

















SUPPLEMENTARY MATERIAL

Field-calibrated Hydrologic Engineering Center – River Analysis System modelling of Krueng Keureuto River flow

Alfiansyah Y. Bantacut¹⁾  , Azmeri Azmeri*¹⁾  , Nina Shaskia¹⁾  ,
Della Amelia¹⁾  , Ziana Ziana¹⁾  , Hafnati Rahmatan²⁾  ,
Eka O. Nugroho³⁾  , Entin Hidayah⁴⁾  

¹⁾ Universitas Syiah Kuala, Engineering Faculty, Civil Engineering Department, Syech Abdur-Rauf No. 7 Darussalam, 23111, Banda Aceh, Indonesia

²⁾ Universitas Syiah Kuala, Faculty of Education and Teacher Training, Biology Department, Tgk. Hasan Krueng Kalee, Darussalam, 23111, Banda Aceh, Indonesia

³⁾ Institut Teknologi Bandung, Faculty of Civil and Environmental Engineering, Water Resources Engineering Research Group, Jl. Ganesha No. 40132, Bandung, Indonesia

⁴⁾ Jember University, Engineering Faculty, Civil Engineering Department, Jl. Kalimantan No. 37 Jember 68121, Indonesia

* Corresponding author

The equations of hydrology process:

$$Q_p = \frac{A \cdot R_e}{3.6 \cdot 0.3T_p + T_{0.3}} \quad (S1)$$

$$T_p = t_g + 0.8t_r \quad (S2)$$

$$t_g = 0.4 + 0.0058L \text{ (for } L \geq 15 \text{ km)} \quad (S3)$$

$$t_g = 0.21L^{0.7} \text{ (for } L < 15 \text{ km)} \quad (S4)$$

$$T_{0.3} = at_g \quad (S5)$$

$$t_r = 0.5t_g \quad (S6)$$

where: Q_p = peak flood discharge ($\text{m}^3 \cdot \text{s}^{-1}$), A = watershed area (km^2), R_e = effective rainfall (mm), T_p = time from the start of the flood to the peak of the flood hydrograph (h), $T_{0.3}$ = time from the start of the flood to $0.3Q_p$ (h), t_g = time of concentration (h), t_r = time unit of rainfall (mm), a = watershed characteristic coefficient of 2, L = length of main river (km).

The baseflow value calculated using the GAMA I method:

$$Q_B = 0.4751A^{0.6444} \cdot D^{0.9430} \quad (S7)$$

where: Q_B = base flow ($\text{m}^3 \cdot \text{s}^{-1}$), A = watershed area (km^2), D = river network density ($\text{km} \cdot \text{km}^{-2}$).

Table S1. Results of steady flow simulation before and after calibration

Specification	Manning's n scenario	Observed discharge ($\text{m}^3 \cdot \text{s}^{-1}$)	Maximum river depth (m)	Deviation from observed depth (%)
Hydrometric measurement	–	16.42	2.70	field condition
Maximum Manning's n	0.15		3.42	26.67
Minimum Manning's n	0.07		2.36	13.33
Calibrated Manning's n	0.08		2.55	5.5

Explanation: Manning's n = roughness coefficient.

Source: own study.

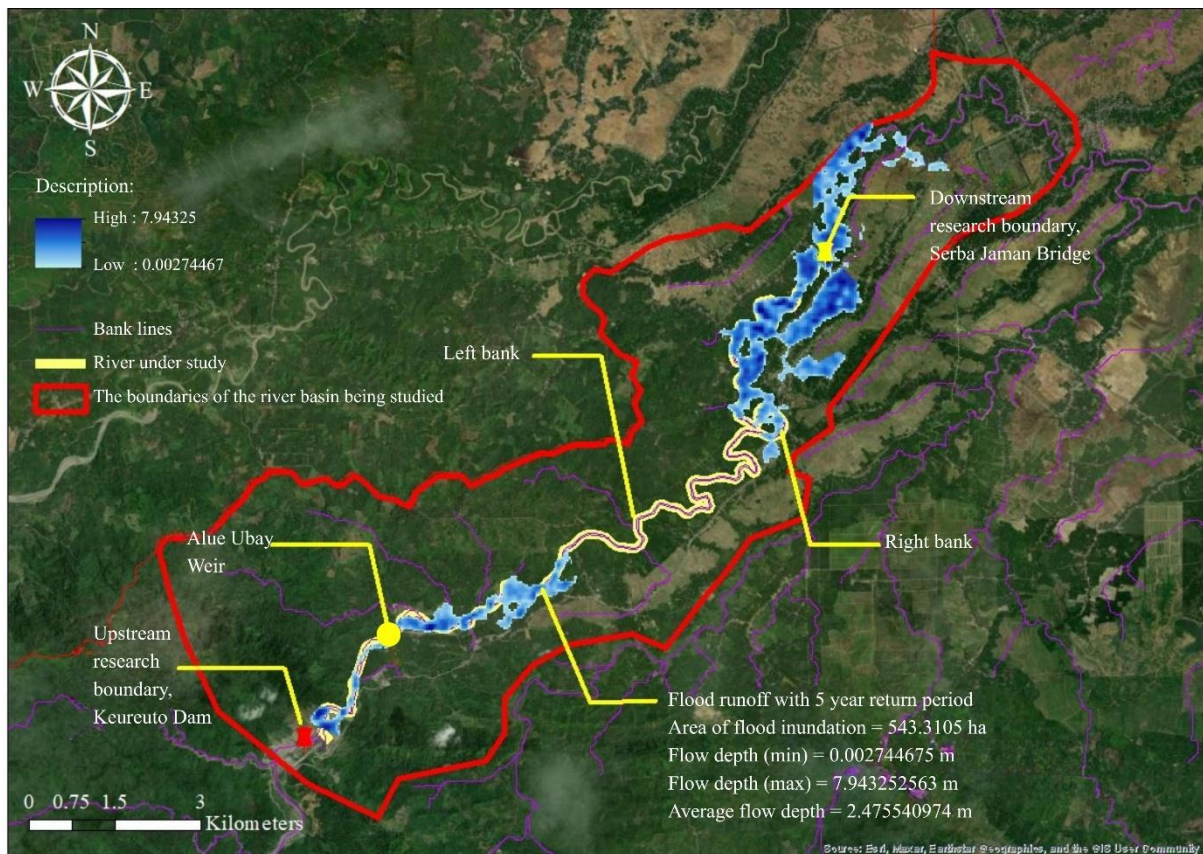


Fig. S1. Flood inundation maps for return periods of 5 years; min = minimum, max = maximum; source: own study

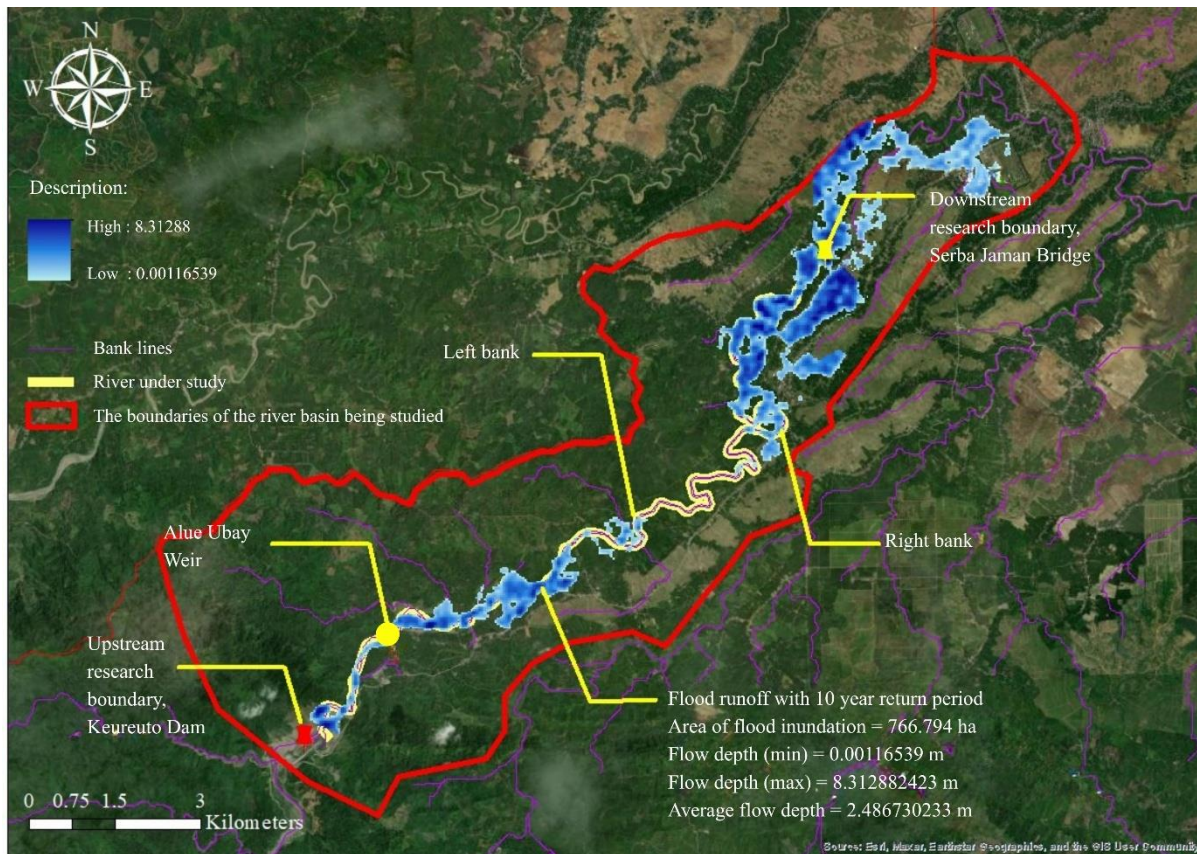


Fig. S2. Flood inundation maps for return periods of 10 years; min, max as in Fig. S1; source: own study

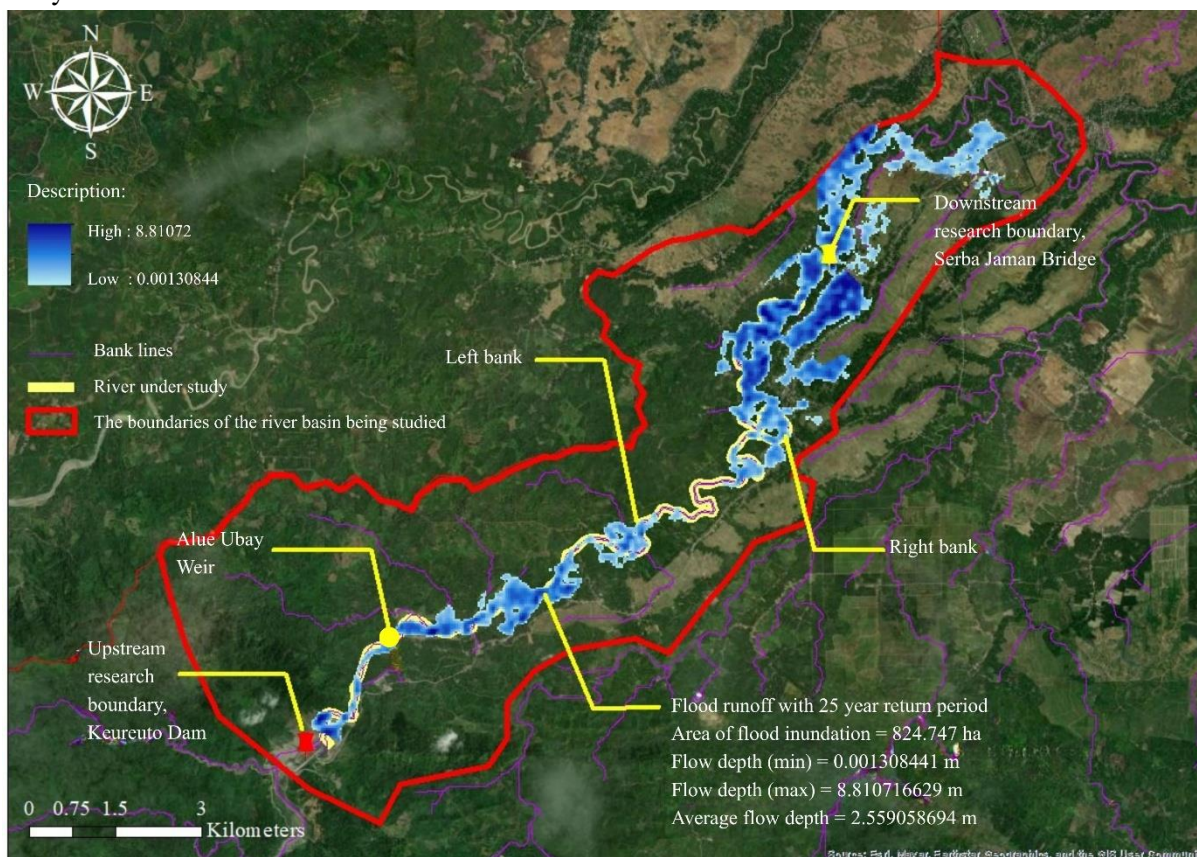


Fig. S3. Flood inundation maps for return periods of 25 years; min, max as in Fig. S1; source: own study

